

MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARDS 1965 A

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WINNIPAUK DAM (A/K/A/ FLOCK PROCESS DAM) CT 00531

NORWALK RIVER BASIN NORWALK, CONNECTICUT

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PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

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SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

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Norwalk, Connecticut					
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ROALD HAESTAD, INC.

CONSULTING ENGINEERS

37 Brookside Road • Waterbury, Conn. 06708 • Tel. 203 753-9800

March 12, 1981

The Department of the Army Corps of Engineers New England Division 424 Trapelo Road Waltham, Massachusetts 02154



Attention: E. P. Gould

Project Management Division

Winnipauk Dam (a/k/a Flock Process Dam)

Norwalk, Connecticut

Gentlemen:

Following field surveys of Winnipauk Dam, we conclude that the dam is too small to qualify under the Federal Dam Inspection Program. Field observations and downstream flood routing computations also indicate that the dam should be classified as "Low" potential hazard.

We are enclosing a brief letter report substantiating our findings.

Very truly yours,

ROALD MAESTAD, INC.

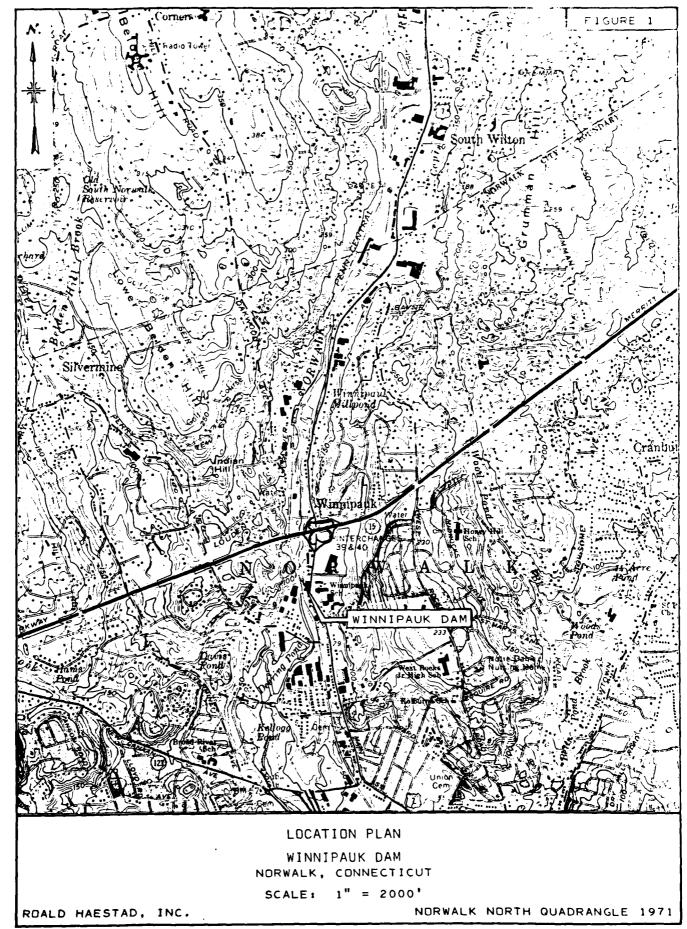
Roald Haestad

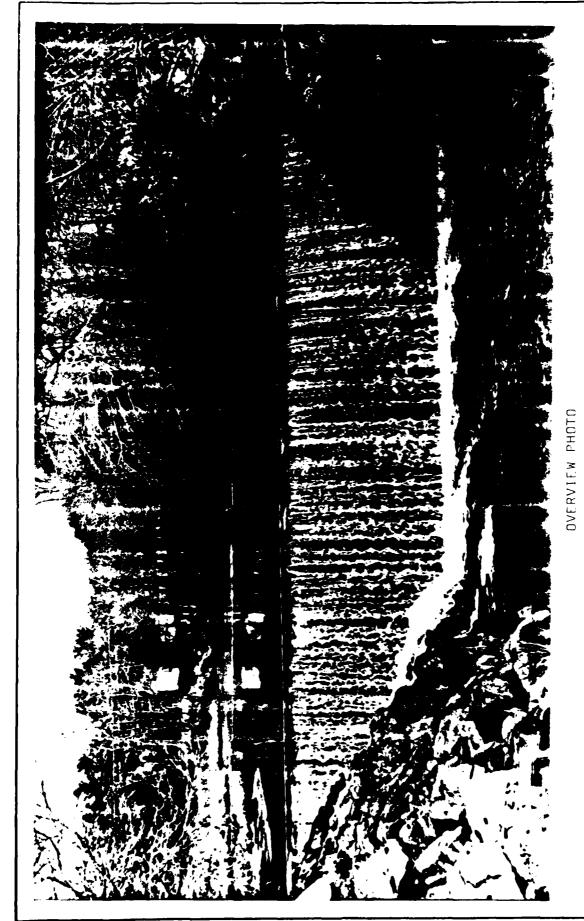
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WINNIPAUK DAM - CT 00531

NATIONAL PROGRAM OF

U.S. ARMY ENGINEER DIV NEW ENGLAND
CORPS OF ENGINEERS
WALTHAM, WASSACHUSETTS

POALD MAESTAD, INC.

INSPECTION OF NON-FED. DAMS

NORWALK RIVER

-2-

DESCRIPTION

The Winnipauk Dam is a run-of-the-river dam consisting of a stone masonry wall with a concrete cap and an upstream earth embankment. The dam has an overflow crest length of 75 feet, a maximum height of 22 feet and outlet works at the left abutment, Overview Photo and Photo 1. The right abutment is a ledge outcrop, Photo 2. A railroad line runs along the right abutment, Photo 1.

There was 0.3 feet of water going over the spillway at the time of inspection, so the condition of the stone masonry wall could not be observed. The concrete weir appeared to be in fair condition. The weir had what looked to be wood planking on both the upstream and downstream edges.

The outlet works at the left abutment, Photo 3, consist of a stone masonry and concrete structure with a 5-1/2' by 5-1/2' sluice gate on the upstream side, Photo 4, which discharges through a partially collapsed 6 foot wide by 4 foot high stone archway at the toe of the left abutment, Photo 3. The sluice gate is obstructed with leaves, sticks and debris and appears to be inoperable. The concrete top of the outlet structure is partially collapsed and the chamber is filled with debris.

The dam has a watershed of 33 square miles and a water surface area of less than two acres. The pond is completely silted in.

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The dam has a watershed of 33 square miles and a water surface area of less than two acres. The pond is completely silted in.

Even assuming an average depth of 20 feet the storage capacity of the impoundment would only be 40 Acre-Feet.

The maximum height of 22 feet and the storage capacity of 40 Acre-Feet are both below the requirements for a "Small" dam. The dam, therefore, does not qualify for inclusion in the Corps of Engineers' Inspection Program.

The downstream channel is the natural river channel, mostly in ledge. A railroad bridge crosses the river approximately 200 feet downstream of the dam, Photo 5. Downstream of the railroad bridge a Corps of Engineers' channel improvement project has provided a channel capacity of about 17,000 cubic feet per second (cfs).

A dam breach analysis was made to determine the hazard classification of the dam. The Corps of Engineers' "'Rule of Thumb' Guidance for Estimating Downstream Dam Failure Hydrographs" was used for the dam breach and flood routing. Two failure conditions were analyzed: 1) a failure with the water level at the top of the dam; and 2) failure with the water at spillway level. Under failure condition 1, the remaining spillway flow, after failure, was added to the dam breach flow.

The peak flood flow for a failure with the water level at the top of the dam would be 7,700 cfs, and failure at spillway level would be 3,000 cfs. Neither flow would exceed the capacity of the downstream channel.

The flood routing shows the dam to be "Low" potential hazard.

APPENDIX A

Photographs

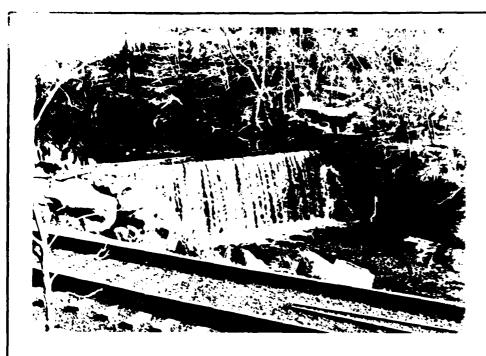


PHOTO NO. 1 SPILLWAY



PHOTO NO. 2

EXPOSED LEDGE AT RIGHT ABUTMENT

U.S ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASSACHUSETTS

ROALD HAESTAD, INC. CONSULTING ENGINEERS WATERBURY, CONNECTICUT

NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS

WINNIPAUK DAM
NORWALK RIVER
NORWALK, CONNECTICUT

CT 00531 17 FEBRUARY 198



PHOTO NO. 3

LEFT ABUTMENT AND OUTLET WORKS



PHOTO NO. 4

OUTLET WORKS FROM UPSTREAM

U.S ARMY ENGINEER DIV NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASSACHUSETTS

ROALD HAESTAD, INC. CONSULTING ENGINEERS WATERBURY, CONNECTICUT

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WINNIPAUK DAM
NORWALK RIVER
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PHOTO NO. 3

LEFT ABUTMENT AND DUTLET WORKS



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U S ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASSACHUSETTS

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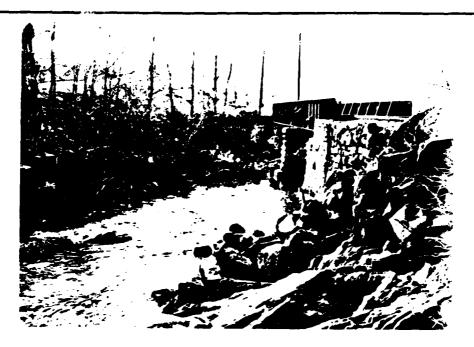


PHOTO NO. 5

RAILROAD BRIDGE LOOKING DOWNSTREAM FROM DAM.



PHOTO NO. 6

RIVER CHANNEL LOOKING DOWNSTREAM FROM RAILROAD BRIDGE.

U.S.ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASSACHUSETTS

ROALD HAESTAD, INC. CONSULTING ENGINEERS WATERBURY, CONNECTICUT

NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS

WINNIPAUK DAM NORWALK RIVER

NORWALK, CONNECTICUT

CT 0053

981

APPENDIX B

Computations

BY SAL DATE 3/4/8/ ROALD HAESTAD, INC. SHEET NO. OF CONSULTING ENGINEERS JOB NO. 49-040 CKD BY JAS DATE 3/5/8/ 37 Brookside Road - Waterbury, Conn. 06708 SUBJECT WINNIPAUK DAM - Dam broach Calculations s= Storage at time of failure with water level attopot dam s = Storage of spillway level + Surcharge Storage S = (Surface Area x Average depth) + (Surface Area X Surcharge S = (ZAcres x /3 feet) + (ZAcres x 7 feet) S = 26 Acre-Feet + 14 Acre-Feet = 40 Acre-Feet Qpi = 8/27 WbVq Yo3/2 = Pook Failure Outflow Wb = Breach Width - 40% of dam length across giver at mid height = 0.4 (75) . 30 feet

Yo: Total height from river bed to pool level at time of failure = zz feet

Qp1 = 9/27 (30) 1/32.2 (22)3/2 = 5,204 use 5,200 cfs

Dom Breach of spillway level:

W6 = 0.4(75) = 30 feet Yo= 15 feet Qp, = 9/27 (30) V32.2 (15)3/2 = 2,930 use 3,000 cfs

Spillway discharge capacity over a 45 foot length.

1. 45 feet (the remainder of the length is assummed to fail with the dam)

Q. CLH 32 = 3 (45) (7) 3/2 = 2,500 cfs

Failure at top of dam = 5,7 + 2,500 = 7,700 cfs

BY SAL DATE 3/4/8/ ROALD HAESTAD, INC. SHEET NO 2 OF 4

CKD BY DLS DATE 3/5/0/ CONSULTING ENGINEERS JOB NO. 049 040

SUBJECT WINNIPAUK DAM-DEPTH OF FLOW

SECTION NUMBER 2

TYPICAL SECTION

Н	W	A	P	S	V	Q
(FI)	(FT)	(SQ-FT)	(FT)	(FT/FT)	(FT/SEC)	(CFS)
1.0	50	цц	0.87	0.0046	2.78	121
2.0	54	95	1.76	0.0046	4,45	422
3.0	57	149	2.59	0.0046	5.76	859
4.0	61	206	3.38	0.0046	6.87	1417
5.0	65	266	4.12	0.0046	7,85	2090
6.0	68	329	4.83	0.0046	8.72	2874
7.0	72	396	5.51	0.0046	9.52	3 768
8.0	76	465	6.15	0.0046	10.25	4763
9.0	82	538	6.60	0.0046	10.75	5785
10.0	88	61.7	7.03	0,0046	11.21	6921
11.0	94	703	7.47	0.0046	11.67	8198
12.0	100	794	7.90	0.0046	12.12	9623
13.0	107	891	8.35	0.004გ	12.57	11201
14.0	113	994	8.79	0.0046	13.01	12941
15.0	119	1104	9.24	0.0046	13.45	14847
16.0	126	1219	9.70	0.0046	13.89	16927

MANNING COEFFICIENT=N=0.0330

REACH OUTFLOW=QP2= 7700 CFS DEPTH OF FLOW=H2= 10.6 FT.

BY -- 7.5.... DATE .5 -- - 5....

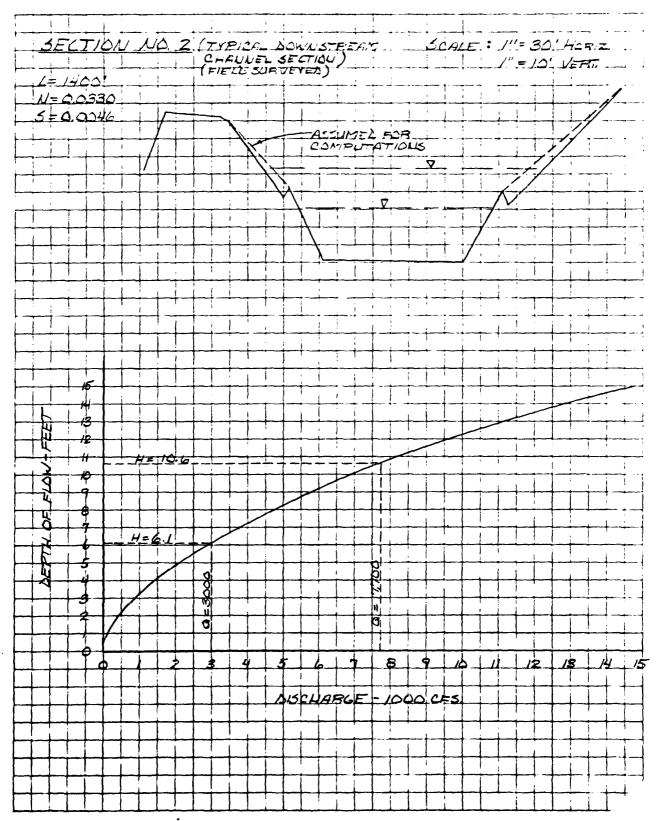
ROALD HAESTAD, INC. SHEET NO. 3 OF 4

CKD BYSALDATE 3-5-81

CONSULTING ENGINEERS 37 Brookside Road - Waterbury, Conn. 06708

JOB NO. 79-474

SUBJECT WANTPAHM DAM - DEPTH OF FLOW



BY SAL DATE 3/4/8/ ROALD HAESTAD, INC. SHEET NO. 4 OF 4

CONSULTING ENGINEERS

CKD BY JUST DATE 3/5/8/ 37 Brookside Road - Waterbury, Conn. 06708 JOB NO. 49-040

SUBJECT WINNIPPUK DAM - Bridges Discharge Capacity

SECTION NO 1: (Railroad Bridge)

Height = 11 feet (Average) Width = 144 feet

Hw/o = "/1 = 1 Entrance Cond: 90° and 15°

Q = 95 cfs/ft × 144 ft = 13,680 cfs

SECTION NO 3: (Perry Avenue)

Height = 15 feet (Average) Width = 78 feet

Hullo = 15/15 = 1 Entrance Cond.: 45°

Q = 160 cfs/4 × 78 ft = 12,480 cfs

SECTION NO 4: (Deering Pand and Kellog Rand Bridge Broad Street)

Height = 11 feet (Average) Width = 157 feet

Hwmax = 6.5 ft to approximate sill level of nearby homes

Hwmax/0 = 451 = 0.59 Entrance Cond. = 45°

Q = 50 cfs/ft × 157 ft = 7,850 cfs



LIMITS OF POTENTIAL FLOODING WINNIPAUK DAM

FAILURE WITH WATER LEVEL AT TOP OF DAM SCALE: 1" = 500'

ROALD HAESTAD, INC.

MARCH 1981